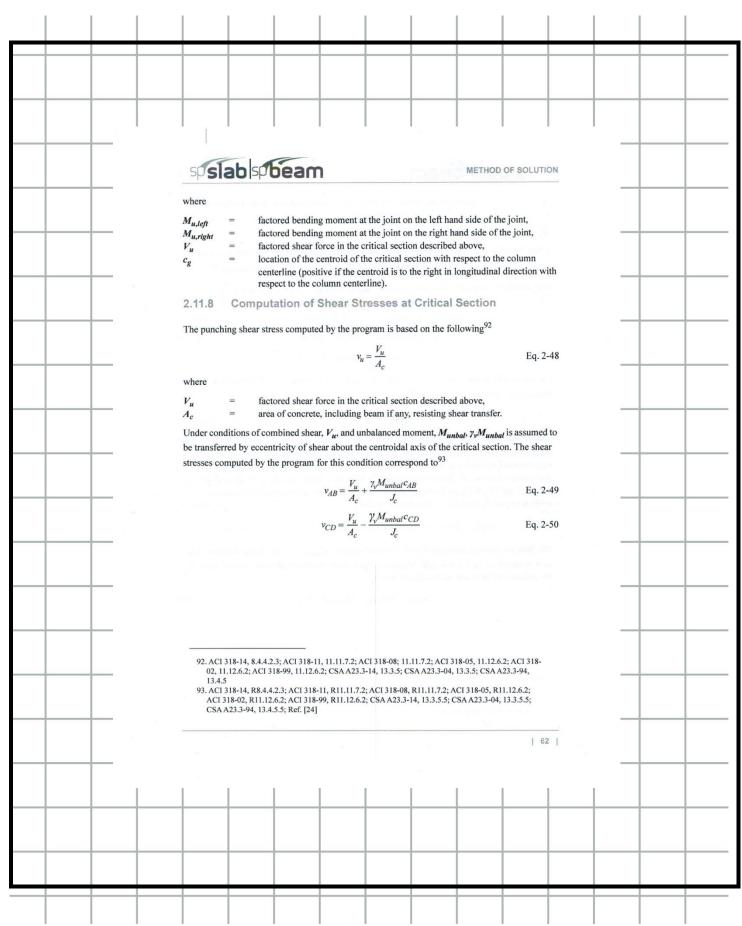
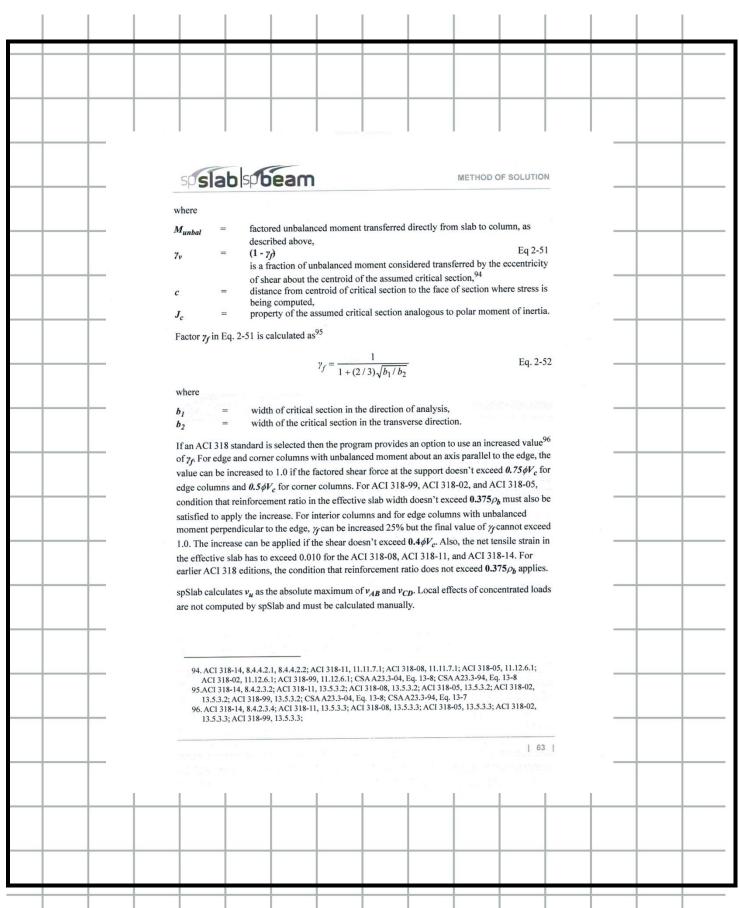
	OMAR S	SHEMY PORTFOI	LIO		
SUBJECT: Unbi	alanced moment, cr				
DATE	PROJECT NO.	PROJECT NA	AME	ENGINEER	PAGE
2024 JAN 18	202402	DEMO PORT	TFOLIO	0.S.	1
1. Study the m	ethod of solution of the so	oftware in use in the in	ivestigation		
	spislab spibeam	and 1.0 for normal density concrete. Refe	ethod of Solution	>	
	determination of $\sqrt{f'_c} \leq 8$ MPa. When the value of d is greater than 3		m the above three		
	The allowable shear stress around dre	rops when waffle slabs are used is compu			
	$\mathbf{v}_{c} = \begin{cases} 2i \\ 0 \\ 0 \\ c \end{cases}$	$2\lambda \sqrt{f_{c}} \qquad \text{for ACI,}$ $0.20 \phi_{c} \lambda \sqrt{f_{c}} \qquad \text{for CSA A23.3-94,}$ $0.19 \phi_{c} \lambda \sqrt{f_{c}} \qquad \text{for CSA A23.3-04.}$	Eq. 2-46		
		ibs defined earlier in this chapter, the allo	owable shear stress is		
	The factored shear force V_u in the critical section (e.g., column centerli superimposed surface dead and live considered open, two 45 degree lines	Factored Shear Force at Critic itical section, is computed as the reaction line for interior columns) minus the self-v load acting within the critical section. If es are drawn from the column corners to the d the self-weight and superimposed surfa- omitted from V_{u} .	n at the centroid of the -weight and any f the section is the nearest slab edge		
	2.11.7 Computation of U	Jnbalanced Moment at Critica	al Section		
		sed for shear transfer, M_{unbab} is compute Moment of the vertical reaction with resp account by			
	Munt	$bal = (M_{u,left} - M_{u,right}) - V_{u} \mathcal{L}_{g}$	Eq. 2-47		
		3.3-04, 13.3.4.3 318-11, 8.13.8; ACI 318-08, 8.13.8; ACI 318-05,	5, 8.11.8; ACI 318-02,		
	8.11.8; ACI 318-99, 8.11.8		61		

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2. Study the code	definition of the solution being	investigated (CL.	13.10.2 CSA	A23.3-2019)
			1 1	
	7.5 Slab reinforcement 13.10 Slab reinforcement			
	13.10.1 General Reinforcement in each direction for two-way slab systems sections but shall be not less than that required by Clause Note: Where strict crack control is a concern, slabs with drop par require additional reinforcement in the negative middle strip regi require additional reinforcement in the negative middle strip regi	7.8.1. nels, particularly in a corrosive environment ion to limit cracking. This additional reinford	;, can cement	
	is not included in the calculation of moment resistance. The reinf more than that required by Clause 7.8.1.	forcement required to limit cracking is gener	rally	
	13.10.2 Shear and moment transfer When gravity load, wind, earthquake, or other lateral force and column, a fraction of unbalanced moment given by			
	$\gamma_f = 1 - \gamma_r$ shall be transferred by flexural reinforcement placed withi	Equatio	in 13.25	
	Note: For exterior supports, including corner columns, Clause 13.	10.3 satisfies this requirement.		
	13.10.3 Exterior columns Reinforcement for the total factored negative moment tra placed within a band width b _b . Temperature and shrinkage Clause 7.8.1 shall be provided in that section of the slab or required by Clause 13.10.9.	e reinforcement determined as specified	d in	
	13.10.4 Spacing Except for portions of slab area that are of cellular or ribbo critical sections shall not exceed the following limits:	ed construction, spacing of reinforceme	ent at	
	Negative reinforcement in the band defined by b_b :	$1.5h_s$, but $s \le 250$ mm $3h_s$, but $s \le 500$ mm	a th	
	Remaining negative moment reinforcement: Positive moment reinforcement:	$3h_{s}$, but $s \leq 500$ mm		
	In the slab over cellular spaces, reinforcement shall be pro	wided as required by Clause 7.8.		
	b_b = width of slab extending 1.5 h_s past the side	les of the column V_{ϵ}		
		M _j Forces to be trai	nsmitted by	
		slab-column co	onnection	
· · · ·	Shear stresses on	ster m		
	critical section due to V _f and (1-y _f)M _f			
	span direction	transfer at slab-column con		
n n	being considered	for interto.		
	Fig. N13.10.2 Shear and moment	t transfer at slab-column con	nections	
	7-21			
				+ + +

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Summarize					
	e unbalanced mom embly.	nent is the moment that	t is drawn to a	column in a column	ı slał
		e-column frame perfect	tly symmetric	with respect to geor	netri
loa	d, there would be i	no rotation at the cente	er column join	t and therefore no n	nome
	uld be drawn into " uld "balance".	the column. The mome	ents on either s	side of the center col	lumn
e) Tha	at being said, if yo	u take that same frame	and make on	e slab/beam span tw	vice a
as t	he other, the mom	nent on either side of th I) of that unbalanced m	e center colun	nn joints would be u	nbal
		ents of force transfer na		be supported by the	COIU
	a. eccentric punc	ching shear γ_v and			
	b. flexure in the s	slab/column joint γ_f .			
d) Fu	ictionally, the unb	alanced moment is rep			
d) Fu	ictionally, the unb				
d) Fu	ictionally, the unb	alanced moment is rep			
d) Fu dia	ictionally, the unb	palanced moment is rep sts itself where the slab/			
d) Fu	ictionally, the unb	alanced moment is rep			
d) Fu dia	nctionally, the unb gram that manifes	palanced moment is rep sts itself where the slab/		passes over the colu	
d) Fu dia	nctionally, the unb gram that manifes 1.28kNm ^H	alanced moment is rep sts itself where the slab/ 3.62kNm		passes over the colu	
d) Fu dia	nctionally, the unb gram that manifes 1.28kNm ^H	3.62kNm ³ -18.79kNm ³	/beam system	passes over the colu [™] [™] [™] [™] [™] [™] [™] -6.63kNm	
d) Fu dia 0.68kNm ⁴	1.28kNm ⁴ 6.26kNm ⁴ 9.62kNm 2.18kNm	3.62kNm ² 3.62kNm ² 3.62kNm ² 30.28kNm -18.79kNm -6.84kNm	/beam system	passes over the colu	
d) Fu dia 0.68kNm ⁴	1.28kNm ⁴ 6.26kNm ⁴ 9.62kNm 2.18kNm	3.62kNm ¹² -18.79kNm -6.84kNm	/beam system	-24.56kNm	
d) Fu dia 0.68kNm ⁴	1.28kNm ⁴ 6.26kNm ⁴ 9.62kNm 2.18kNm	3.62kNm ² 3.62kNm ² 3.62kNm ² 30.28kNm -18.79kNm -6.84kNm	/beam system	-24.56kNm	
d) Fu dia 0.68kNm ⁴	1.28kNm ⁴ 6.26kNm ⁴ 9.62kNm 2.18kNm	3.62kNm ² 3.62kNm ² 3.62kNm ² 30.28kNm -18.79kNm -6.84kNm	/beam system	-24.56kNm	
d) Fu dia 0.68kNm ⁴ 0.29kNm 1	1.28kNm 1.28kNm 1.28kNm 1.28kNm 9.62kNm 2.18kNm 5.3kNBkNm 6.1	3.62kl/m -18.79kl/m -18.65kl/m -18.65kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -18.79kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -19.70kl/m -1	/beam system	Passes over the colu -24.56kNm 13.71kkm -10.85kNm	
d) Fu dia 0.68kNm ⁴	1.28kNm ⁴ 6.26kNm ⁴ 9.62kNm 2.18kNm	3.62kNm ² 3.62kNm ² 3.62kNm ² 30.28kNm -18.79kNm -6.84kNm	/beam system	-24.56kNm	
d) Fui dia 0.68kNm [#] 0.29kNm 1	1.28kNm ⁴ 1.28kNm ⁴ 1.28kNm ⁴ 53kA8kNm 53kA8kNm 6.1	3.62kNm -18.79kNm -18.65kNm -18.65kNm	/beam system	Passes over the colu -24.56kNm 13.71kkm -10.85kNm	

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2.8. Bottom Bar Devicem Short Bars Stort Bars															
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							U2 All	OK	1.24	38.97	0.00	U1 All	OK		
			8.800	5.58	-159.61	-86.38	U2 All	OK	1.24	38.97	0.00	U1 All			
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Mo is t the Suj	left span 128.74 - 37.06 = 165 right span ment left of the s he end of the spa left ment right of the port is the start in on the right	3 Column support an on	x ft 9.500 10.500 3.000 3.975 5.250 6.525 9.500 10.500 0.000 0.750 1.000 0.750 1.000 4.401 5.417	As.top in ² 5.58 5.58 5.58 3.72 3.72 3.72 3.72 3.72 3.72 3.72 3.72	• Mn- k-ft -159.61 -159.61 -117.01 -117.01 -117.01 -117.01 -117.01 -117.01 -117.01 -117.01 -117.01 -117.01 -1159.61	Mu- k-ft -102.99 (-128.74) 0.72 0.00 -0.62 -1.34 -3.23 -6.71 -25.75	Comb Pat U2 All U2 All U2 S1 U2 S1 U2 Odd U2 Odd U2 Odd U2 Odd	OK OK OK OK OK	A _{s,bot} in ² 1.24 1.24 3.72 3.72 3.72 3.72	ФМ _n + <u>k-ft</u> 38.97 38.97 117.01 117.01	M _u + k-ft 0.00 0.00 0.00 0.52	Comb Pat U1 All U1 All U1 All U3 S3	OK OK	_
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Mo is t the Suj	right span ment left of the s he end of the spa left ment right of the oport is the start	3 Column support an on	3.000 3.975 5.250 6.525 9.500 10.500 0.000 0.750 1.000 4.401	3.72 3.72 3.72 3.72 3.72 3.72 3.72 5.58 5.58	-117.01 -117.01 -117.01 -117.01 -117.01 -117.01	-0.62 -1.34 -3.23 -6.71	U2 Odd U2 Odd U2 Odd	OK						
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is t the Mo su spa	right span ment left of the s he end of the spa left ment right of the oport is the start	3 Column support an on	6.525 9.500 10.500 0.000 0.750 1.000 4.401	3.72 3.72 3.72 5.58 5.58	-117.01 -117.01 -117.01	-6.71		OK	3.72 3.72	117.01 117.01	1.02	U3 Even U1 All	OK	
is t the Mo su spa	right span ment left of the s he end of the spa left ment right of the oport is the start	3 Column support an on	9.500 10.500 0.000 0.750 1.000 4.401	3.72 3.72 5.58 5.58	-117.01 -117.01		U2 Odd	OK	3.72	117.01	0.00	U1 All	OK	
is t the Mo su spa	ment left of the s he end of the spa e left ment right of the oport is the start	support an on	0.000 0.750 1.000 4.401	5.58 5.58			U2 All	OK	3.72	117.01	0.00	U1 All	OK	
is t the Mo su spa	ment left of the s he end of the spa e left ment right of the oport is the start	support an on	0.750 1.000 4.401	5.58	150 61	-37.06	U2 All		3.72	117.01	0.00	U1 All		
is t the Mo su spa	ment left of the s he end of the spa e left ment right of the oport is the start	support an on	1.000 4.401			-204.69	U2 All		3.72	115.25	0.00	U1 All		
is t the Mo su spa	he end of the spa left ment right of the oport is the start	an on	4.401		-159.61	-166.24	U2 All U2 All		3.72 3.72	115.25 115.25	0.00	U1 All U1 All	OK	
the Mo su spa	left ment right of the port is the start			5.58	-159.61 -159.61	-153.92	U2 All	OK	3.72	115.25	0.00	U1 All	OK	
Mo suj spa	ment right of the port is the start			2.79	-84.87	0.00	U1 All	OK	3.72	115.25	16.72	U2 All	OK OK	
su spa	port is the start		7.272 8.287	2.79 0.00	-84.87	0.00	U1 All U1 All	OK OK	3.72 3.72	115.25 115.25	54.72 71.13	U2 All U2 All	OK	
su spa	port is the start		8.729	0.00	0.00	0.00	U1 All	OK	3.72	115.25	77.31	U2 All	OK	
spa			11.750 13.158	0.00	0.00		U1 All U1 All	OK	3.72	115.25 115.25	103.74 106.69	U2 All U2 All	OK	
	an on the right	of the	15.354	0.00	0.00		U1 All	OK	3.72	115.25	99.34	U2 All	OK	
			15.796	0.00	0.00		U1 All	OK	3.72	115.25	96.10 86.62	U2 All U2 All	OK OK	
	204.69 - 51.17 =	-255.86	16.796 18.667	1.24	-38.22		U1 All U1 All	OK OK	3.72	115.25 115.25	60.80	U2 All	OK	
	100		19.667	2.17	-64.59	0.00	U1 All	OK	3.72	115.25	42.66	U2 All	OK	
			23.083 23.500	2.17 2.17	-64.59 -64.59	-56.35 -72.95	U2 S3 U2 All	OK	3.72	115.25 115.25	0.00	U1 All U1 All	OK	
	right span	Middle	0.000	3.72	-117.01	-51.17	U2 All		2.79	87.60	0.00	U1 All		
		_	1.000	3.72	-117.01	-38.48 0.00	U2 All U1 All	OK OK	2.79	87.60 87.60	0.00	U1 All U2 All	OK OK	
			5.350 6.350	3.72 0.00	-117.01 0.00		U1 All	OK	2.79	87.60	24.75	U2 All	OK	
			8.729	0.00	0.00		U1 All	OK	2.79	87.60	51.54	U2 All U2 All	OK	
			11.750 13.158	0.00	0.00		U1 All U1 All	OK OK	2.79	87.60 87.60	69.16 71.13	U2 All	OK	
			15.354	0.00	0.00	0.00	U1 All	OK	2.79	87.60	66.23	U2 All	OK	
			18.225 19.225	0.00	0.00		U1 All	OK OK	2.79	87.60 87.60	45.22 34.02	U2 All U2 All	OK OK	
			23.083	4.34	-136.43		U2 S3	OK	2.79	87.60	0.00	U1 All	OK	
			23.500	4.34	-136.43	-18.24	U2 All	- 52	2.79	87.60	0.00	U1 All		
		4 Column	0.000	2.17	-64.59	-49.68	U2 S3		0.00	0.00	0.00	U1 All		
		11 (A 11)	0.417	2.17	-64.59	-46.54	U2 S3	OK	0.00	0.00	0.00	U1 All	OK	
			3.597 4.597	2.17 1.24	-64.59 -38.22		U2 All U2 Odd	OK OK	0.00	0.00	0.00	U1 All U1 All	OK OK	
			4.850	1.24	-38.22	-27.83	U2 Odd	OK	0.00	0.00	0.00	U1 All	OK	
			6.750	1.24	-38.22 -38.22		U2 Odd U2 Odd		0.00	0.00	0.00	U1 All U1 All	OK OK	
			8.650 8.903	1.24 1.24	-38.22		U2 Odd		0.00	0.00	0.00	U1 All	OK	
			9.903	2.17	-64.59		U2 All	OK	0.00	0.00	0.00	U1 All U1 All	OK OK	
			10.550 11.550	2.17 2.79	-64.59 -81.07		U2 All U2 All	OK OK	0.00	0.00	0.00	U1 All	OK	
			13.083	2.79	-81.07	-53.34	U2 All	OK	0.00	0.00	0.00	U1 All	OK	
			13.500	2.79	-81.07	-56.80	U2 S4	< 3	0.00	0.00	0.00	U1 All		

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Span Strip x A. M. Comb Pit Status And Other Pit Status Middle 0.000 4.34 114.84 12.42 U2.83	Middle 0 11 1 11 1 11 1 11 1 11 1 12 1 13 1 14 1 15 Column 11 1 12 2 13 1 14 1 15 Column 16 Column 11 1 12 2 14 1 15 Column 16 Column	ft im 0.000 4.34 0.0417 4.34 2.203 4.33 2.203 4.33 2.203 4.33 3.203 0.93 4.850 0.93 6.750 0.93 10.297 0.91 11.297 2.11 13.083 2.11 0.000 2.7 0.417 2.7 0.433 2.7 5.833 1.5 8.354 0.0 9.354 0.00 9.364 0.00 18.021 0.00 18.021 0.00 18.021 0.00 18.023 0.00 18.024 0.00 18.025 0.00 18.021 0.00 18.023 0.00 19.571 2.4 23.092 4.6 27.500 4.6	● ●M _n - 2 k-ft 4 -118.45 4 -118.45 4 -118.45 4 -118.45 4 -118.45 4 -118.45 4 -118.45 4 -118.45 4 -118.45 3 -28.99 3 -28.99 3 -28.99 3 -28.99 3 -28.99 3 -28.99 3 -28.99 3 -28.99 3 -28.99 3 -28.99 3 -28.99 3 -28.99 -81.07 -81.07 9 -81.07 9 -81.07 9 -81.07 9 -81.07 9 -81.07 9 -81.07 9 -81.07 0 0.00 0 0.00 <t< th=""><th>Mu- k-ft -12.42 -11.64 -8.86 -7.89 -6.96 -6.90 -7.59 -9.03 -10.33 -10.33 -13.34 -14.20 -88.08 -82.30 -76.61 0.00 0.000 0.000 0.000 0.000 0.000 0.000</th><th>U2 S3 U2 S3 U2 All U2 All U2 All U2 Odd U2 Odd U2 All U2 All U2 All U2 All U2 All U2 All U2 All U2 All U2 All U2 All U1 All</th><th></th><th>in² 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.</th><th>k-ft 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29</th><th>Mu+ k-ft 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.</th><th>Comb Pat U1 All U1 All U2 All</th><th></th><th></th><th></th></t<>	Mu- k-ft -12.42 -11.64 -8.86 -7.89 -6.96 -6.90 -7.59 -9.03 -10.33 -10.33 -13.34 -14.20 -88.08 -82.30 -76.61 0.00 0.000 0.000 0.000 0.000 0.000 0.000	U2 S3 U2 S3 U2 All U2 All U2 All U2 Odd U2 Odd U2 All U2 All U2 All U2 All U2 All U2 All U2 All U2 All U2 All U2 All U1 All		in ² 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.	k-ft 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29 76.29	Mu+ k-ft 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.	Comb Pat U1 All U1 All U2 All			
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4 38.50 38.50 7.59 39.27 U2 All 0.600 0.749 2.790 5 40.50 40.50 7.59 81.55 U2 Odd 0.542 1.435 2.790 6 40.50 40.50 7.59 71.2 U2 Odd 0.600 0.133 1.550 2.12. Punching Shear Around Columns 2.12.1. Critical Section Properties Support Type b1 b2 b0 dawg CG cmah Ac Jc 1 Rect (31.59) 102.34 7.59 0.00 (15.79) 776.25 1.1623e+005 2 Rect 31.59 19.59 102.34 7.59 0.00 15.79 776.25 1.1623e+005
5 40.50 40.50 7.59 81.55 1/2 Odd 0.542 1.435 2.790 6 40.50 40.50 7.59 7.12 U2 Odd 0.600 0.133 1.550 2.12. Punching Shear Around Columns 2.12.1. Critical Section Properties Support Type b1 b2 b6 dawg CG ctrant Ac J6 1 Rect (31.59) 102.34 7.59 0.00 (15.79) 776.25 1.1623e+005 2 Rect 31.59 19.59 102.34 7.59 0.00 15.79 15.79 776.25 1.1623e+005
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5 Rect 31.59 19.59 102.34 7.59 0.00 15.79 15.79 776.25 1.1623e+005
6 Rect 19.59 19.59 78.34 7.59 0.00 9.79 9.79 594.21 39412
2.12.2. Punching Shear Results
Support Vu vu Munb Comb Patt yv vu ΦVc kip psi k-ft psi psi </td
1 <u>35.25</u> 45.4 <u>-48.79</u> U2 All 0.458 <u>81.9</u> 212.4
2 104.56 134.7 90.06 U2 All 0.458 202.0 212.4
3 60.11 112.7 -31.26 U2 All 0.400 158.5 212.4
4 44.71 83.8 39.27 U2 All 0.400 141.4 212.4 5 73.79 95.1 -79.35 U2 All 0.458 154.4 212.4
5 73.79 95.1 -79.35 U2 All 0.458 154.4 212.4 6 40.07 67.4 5.33 U2 All 0.400 73.8 212.4
2.13. Integrity Reinforcement at Supports Notes: The sum of bottom reinforcement crossing the perimeter of the support on all sides shall not be less than the below listed values.
Support V _{se} A _{sb} kip in ²
1 29.830 0.994
2 107.444 3.581
3 61.108 2.037
4 45,473 1.516 5 75 807 0.530
5 75.897 2.530 6 35.288 1.176
2.14. Material TakeOff
2.14.1. Reinforcement in the Direction of Analysis
2.14.1. Reinforcement in the Direction of Analysis Top Bars 1526.8 lb <=> 15.42 lb/ft <=> 1.008 lb/ft²
2.14.1. Reinforcement in the Direction of Analysis Top Bars 1526.8 lb <=> 15.42 lb/ft <=> 1.008 lb/ft² Bottom Bars 1273.4 lb <=> 12.86 lb/ft <=> 0.841 lb/ft²
2.14.1. Reinforcement in the Direction of Analysis Top Bars 1526.8 lb <=> 15.42 lb/ft <=> 1.008 lb/ft ² Bottom Bars 1273.4 lb <=> 12.86 lb/ft <=> 0.841 lb/ft ² Stirrups 0.0 lb <=> 0.00 lb/ft <=> 0.000 lb/ft ²
2.14.1. Reinforcement in the Direction of Analysis Top Bars 1526.8 lb <=> 15.42 lb/ft <=> 1.008 lb/ft ² Bottom Bars 1273.4 lb <=> 12.86 lb/ft <=> 0.841 lb/ft ² Stirrups 0.0 lb <=> 0.00 lb/ft <=> 0.000 lb/ft ² Total Steel 2800.2 lb <=> 28.28 lb/ft <=> 1.849 lb/ft ²
2.14.1. Reinforcement in the Direction of Analysis Top Bars 1526.8 lb <=> 15.42 lb/ft <=> 1.008 lb/ft ² Bottom Bars 1273.4 lb <=> 12.86 lb/ft <=> 0.841 lb/ft ² Stirrups 0.0 lb <=> 0.00 lb/ft <=> 0.000 lb/ft ² Total Steel 2800.2 lb <=> 28.28 lb/ft <=> 1.849 lb/ft ²
2.14.1. Reinforcement in the Direction of Analysis Top Bars 1526.8 lb <=> 15.42 lb/ft <=> 1.008 lb/ft ² Bottom Bars 1273.4 lb <=> 12.86 lb/ft <=> 0.841 lb/ft ² Stirrups 0.0 lb <=> 0.00 lb/ft <=> 0.000 lb/ft ² Total Steel 2800.2 lb <=> 28.28 lb/ft <=> 1.849 lb/ft ²
2.14.1. Reinforcement in the Direction of Analysis Top Bars 1526.8 lb <=> 15.42 lb/ft <=> 1.008 lb/ft ² Bottom Bars 1273.4 lb <=> 12.86 lb/ft <=> 0.841 lb/ft ² Stirrups 0.0 lb <=> 0.00 lb/ft <=> 0.000 lb/ft ² Total Steel 2800.2 lb <=> 28.28 lb/ft <=> 1.849 lb/ft ²
2.14.1. Reinforcement in the Direction of Analysis Top Bars 1526.8 lb <=> 15.42 lb/ft <=> 1.008 lb/ft ² Bottom Bars 1273.4 lb <=> 12.86 lb/ft <=> 0.841 lb/ft ² Stirrups 0.0 lb <=> 0.00 lb/ft <=> 0.000 lb/ft ² Total Steel 2800.2 lb <=> 28.28 lb/ft <=> 1.849 lb/ft ²
2.14.1. Reinforcement in the Direction of Analysis Top Bars 1526.8 lb <=> 15.42 lb/ft <=> 1.008 lb/ft ² Bottom Bars 1273.4 lb <=> 12.86 lb/ft <=> 0.841 lb/ft ² Stirrups 0.0 lb <=> 0.00 lb/ft <=> 0.000 lb/ft ² Total Steel 2800.2 lb <=> 28.28 lb/ft <=> 1.849 lb/ft ²
2.14.1. Reinforcement in the Direction of Analysis Top Bars 1526.8 lb <=> 15.42 lb/ft <=> 1.008 lb/ft ² Bottom Bars 1273.4 lb <=> 12.86 lb/ft <=> 0.841 lb/ft ² Stirrups 0.0 lb <=> 0.00 lb/ft <=> 0.000 lb/ft ² Total Steel 2800.2 lb <=> 28.28 lb/ft <=> 1.849 lb/ft ²
2.14.1. Reinforcement in the Direction of Analysis Top Bars 1526.8 lb <=> 15.42 lb/ft <=> 1.008 lb/ft ² Bottom Bars 1273.4 lb <=> 12.86 lb/ft <=> 0.841 lb/ft ² Stirrups 0.0 lb <=> 0.00 lb/ft <=> 0.000 lb/ft Total Steel 2800.2 lb <=> 28.28 lb/ft <=> 1.849 lb/ft ²

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5. Verify the output of the software and the method of solution Joint support 2 (Col. G/3): Envelope at the joint: From the left = @10.5 = -128.74 - 37.06 = -165.8 k.ft M_{LEFT} From the right = @0.0 = -204.69 - 51.17 = -255.86 k.ft M_{RIGHT} + + Unbatanced moment, $M_{unb} = (M_{LEFT} - M_{RIGHT}) - V_U \cdot C_g$ -165.8255.86 - $V_U(0) = 90.06 k.ft$ Support 2: b ₁ = 31.59 in, b ₂ = 19.59 in, b ₀ = 2(b ₁ + b ₂) = 102.34 in d _{awg} = 7.59 in, $A_C = 776.25 in^2$, $J_C = 1.1632 * 10^{15} in^4$ $V_U = \frac{104.56}{776.25} * 1000 = \frac{134.7}{12} psi$ $C_{AB} = 15.79 in$ $V_{AB} = \frac{Y_U * M_{unb} * (1000 * 12) * c_{AB}}{1.1632 * 10^{15}} = 67.2 psi$ $V_{AB} = \frac{0.458 * 90.06}{V_U = 134.7 + 67.2 = 202 psi$					
Joint support 2 (Col. G/3): Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2">Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"Colspan="2"C	5. Verify the outp	out of the software a	und the method	of solution	
$C_{g} = C$ $From the left = @10.5 = -128.74 - 37.06 = -165.8 k. ft M_{LEFT}$ $From the right = @0.0 = -204.69 - 51.17 = -255.86 k. ft M_{RIGHT}$ $Unbalanced moment, M_{unb} = (M_{LEFT} - M_{RIGHT}) - V_U \cdot C_g$ $-165.8 - 255.86 - V_U(0) = 90.06 k. ft$ $Support 2:$ $b_1 = 31.59 in, b_2 = 19.59 in, b_o = 2(b_1 + b_2) = 102.34 in$ $d_{avg} = 7.59 in, A_C = 776.25 in^2, J_C = 1.1632 * 10^{15} in^4$ $V_U = \frac{104.56}{776.25} * 1000 = 134.7 psi$ $c_{AB} = 15.79 in$ $v_{AB} = \frac{\gamma_v * M_{unb} * (1000 * 12) * c_{AB}}{J_C}$ $V_{AB} = \frac{0.458 * 90.06 * (1000 * 12) * 15.79}{1.1632 * 10^{15}} = 67.2 psi$	Joint support	<u>2 (Col. G/3):</u>			
$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	Envelop	e at the joint:			$C_{-}=0$
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	Fre	om the left $= @10$.	5 = -128.74 -	37.06 = -165.8 k.f	
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	Fror	$n \ the \ right = @0.0$	0 = -204.69 - 100	51.17 = -255.86 k.f	t M _{RIGHT} +
Support 2: $b_1 = 31.59 in,$ $b_2 = 19.59 in,$ $b_o = 2(b_1 + b_2) = 102.34 in$ $d_{avg} = 7.59 in,$ $A_c = 776.25 in^2,$ $J_c = 1.1632 * 10^{15} in^4$ $V_U = \frac{104.56}{776.25} * 1000 = 134.7 psi$ $C_{AB} = 15.79 in$ $C_{AB} = 15.79 in$ $v_{AB} = \frac{\gamma_v * M_{unb} * (1000 * 12) * c_{AB}}{J_c}$ J_c $I_{AB} = 67.2 psi$		Unbalanced mom	ent, $M_{unb} = (M$	$_{LEFT} - M_{RIGHT}) - V_U.$	<i>C_g</i> /
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$		-165.8-	$-255.86 - V_U$	(0) = 90.06 k.ft	Critical section
$\begin{aligned} d_{avg} &= 7.59 \text{ in}, A_c = 776.25 \text{ in}^2, J_c = 1.1632 * 10^{15} \text{ in}^4 \\ V_U &= \frac{104.56}{776.25} * 1000 = 134.7 \text{ psi} \\ c_{AB} &= 15.79 \text{ in} \\ lower &= 15.79 \text{ in} \\ lower &= 15.79 \text{ in} \\ lower &= 15.79 \text{ in} \\ v_{AB} &= \frac{\gamma_v * M_{unb} * (1000 * 12) * c_{AB}}{J_c} \\ lower &= 0.458 * 90.06 * (1000 * 12) * 15.79 \\ lower &= 1.1632 * 10^{15} \\ lower &= 67.2 \text{ psi} \end{aligned}$	Support	t 2:			
$V_{U} = \frac{104.56}{776.25} * 1000 = 134.7 psi$ $c_{AB} = 15.79 in$ $v_{AB} = \frac{\gamma_{v} * M_{unb} * (1000 * 12) * c_{AB}}{J_{C}}$ $v_{AB} = \frac{0.458 * 90.06 * (1000 * 12) * 15.79}{1.1632 * 10^{15}} = 67.2 psi$	<i>b</i> ₁ :	$= 31.59 in, b_2$	= 19.59 in,	$b_o = 2(b_1 + b_2) = 10$	2.34 in
$v_{AB} = \frac{\gamma_{v} * M_{unb} * (1000 * 12) * c_{AB}}{J_{c}}$ $v_{AB} = \frac{0.458 * 90.06 * (1000 * 12) * 15.79}{1.1632 * 10^{15}} = 67.2 \text{ psi}$	d	avg = 7.59 in, A	$c = 776.25 in^2,$	$J_c = 1.1632 * 10^{15}$	⁵ in ⁴
$v_{AB} = \frac{\gamma_{v} * M_{unb} * (1000 * 12) * c_{AB}}{J_{c}}$ $v_{AB} = \frac{0.458 * 90.06 * (1000 * 12) * 15.79}{1.1632 * 10^{15}} = 67.2 \text{ psi}$		$V_{tr} =$	104.56 * 1000	= 134.7 nsi	
$v_{AB} = \frac{\gamma_{v} * M_{unb} * (1000 * 12) * c_{AB}}{J_{C}}$ $v_{AB} = \frac{0.458 * 90.06 * (1000 * 12) * 15.79}{1.1632 * 10^{15}} = 67.2 \text{ psi}$					
$v_{AB} = \frac{0.458 * 90.06 * (1000 * 12) * 15.79}{1.1632 * 10^{15}} = 67.2 \text{ psi}$					
$v_{AB} = \frac{1.1632 * 10^{15}}{1.1632 * 10^{15}} = 67.2 psi$		$v_{AB} = -$	$\frac{\gamma_v * M_{unb} * (100)}{J_c}$	$(0 * 12) * c_{AB}$	
1.1632 * 1015		0.458 * 9		-h//nc	
$v_u = 134.7 + 67.2 = 202 \ psi$					
		v_u :	= 134.7 + 67.2	= 202 psi	